

Possibility of improvement of potentiodynamic method for monitoring corrosion rate of steel reinforcement in concrete

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Abstract. Quantitative data on corroding steel reinforcement in reinforced concrete structures are undoubtedly very useful for evaluation of their service life and timely repairs. The method of electrode potential measurement is a convenient and simple test for this purpose, but it provides no quantitative data on corrosion rate and only information regarding active or passive state of steel reinforcement can be obtained. We show here the possibility of obtaining quantitative data on degree of corrosion of steel reinforcement by a potentiodynamic method. The developed method is based on experimentally estimated mathematical relation between the results of potentiodynamic method and degree of corrosion of steel reinforcement. It is possible to calculate the degree of corrosion of steel reinforcement using this mathematical relation and the measured values of current density by the potentiodynamic method.

Keywords. Steel reinforcement; corrosion rate; potentiodynamic method; electrical resistance method.

1. Introduction

Corrosion monitoring of the state of steel reinforcement in concrete is undoubtedly very useful. It can provide very important information relating to evaluation of service life of reinforced concrete structures. The conditions and processes that occur on steel reinforcement in concrete are electrochemical in nature. This fact makes possible the use of physicochemical methods like the method of electrode potential (Andrade and Gonzales 1978; Alekseev *et al* 1985; Andrade *et al* 1995) for monitoring the state of the steel reinforcement. However, these methods can only give information about the electrochemical state of the reinforcement, if it is passive or active. This represents a significant drawback of these methods. The method of linear polarization which is able to measure the corrosion rate is one exception (Stern and Geary 1957; Stern and Weisert 1959; Pequin and Chevalier 1971; Page and Havdahl 1985; Broomfield 1997). But even this method has several limitations:

(i) Choice of values of the constant B in the Stern–Gear equation

$$I_{\text{corr}} = \frac{B}{R_p}, \quad (1)$$

where I_{corr} is corrosion current and R_p the polarization resistance. The value of the constant B is dependent upon the magnitude of kinetic coefficients of the anodic and cathodic reactions of the corrosion. It may vary from 26 to 52 mV. The deciding condition is if the steel is passive or active (Broomfield 1997). This condition makes the

choice of the value of the constant B very problematic, because the state of the steel may change. How to realize this is a problem.

(ii) The method detects the instantaneous corrosion rate.

(iii) The results of the method may have errors of 10 to 100, especially at low corrosion rate (Broomfield 1997).

(iv) When corrosion is concentrated on the top of the steel bars or, if the steel bars are close together or deep within the concrete the device may only send current to the top steel. Both of these errors mean that the best accuracy that can be expected from a linear polarization device is a factor of two to four (Broomfield 1997).

A dominant consequence of reinforcement corrosion is decrease in cross-section of the steel bars. This fact is used in the method of electrical resistance (MER). The method is based on the subsequent increase in electrical resistance of the corroding reinforcement, which is measured electrically. Several techniques of MER were developed and used (Freedman *et al* 1958; Schippa 1967). An improved MER technique is MER based on so-called corrosion sensor. The sensor is a properly modified specimen of reinforcement. The corrosion sensor is embedded in cement composite test specimens or the reinforced concrete structure and enables measurement of corrosion rate with sufficient sensitivity. It is important that various disturbing effects like ambient temperature changes, electrical tensions and others are excluded. Corrosion is indicated by increase in electrical resistance. Moreover, by means of the contingent equations and results of the MER, it is possible to calculate some characteristics of the corroding reinforcement, like decrease

of section, depth of corroded layer and loss of steel material, and to estimate the corrosion rate. More detailed information about the MER is given elsewhere (Živica 1997).

Comparison of the two methods shows that MER seems to be better than the method of linear polarization. One exception is limitation (ii), which is valid even for MER.

While evaluating the two methods the fact that the high heterogeneity of chloride-induced corrosion should be taken into consideration. This requirement is documented by the found values of variation coefficient of the MER results under the chloride-induced corrosion (Živica 1991).

It has been found that the values of variation coefficient may vary in a broad range, moreover depending on time of the exposure of the concrete. After 15 days of exposure it was 58%, and after 170 days only 10%. This shows high dispersion of the MER results especially in the initial stage of corrosion. For comparison under the same conditions, the results of stationary electrode potential method show a value of variation coefficient of around 8%. The high dispersion of MER results observed is of pitting corrosion, which is a well known phenomenon of chlo-

ride-induced corrosion. Its development is heterogeneous, especially in the initial stage. With increased exposure the heterogeneity decreases as a result of the gathering of individual pitting points on the surface of the corroding steel. This undoubtedly affects the accuracy of measurement. Better accuracy of measurement may be expected in measurement of uniform corrosion (e.g. carbonation), where the effect of formation of pitting points is excluded and instead, uniform attack of the steel occurs.

The purpose of the present study was improvement of the potentiodynamic method (PDM) for the measurement of corrosion rate.

The improvement of PDM was based on the following: (i) to find the relationship between the proper PDM characteristic of the corroding reinforcement and the increase of corrosion sensor, ΔR measured by MER under the same conditions and (ii) to obtain a mathematical expression of the found relationship, giving the possibility of calculating ΔR using the results of PDM, and comparing the calculated ΔR values with those measured by MER.

2. Experimental

(i) Experimental study of steel reinforcement corrosion induced by chlorides by means of MER and PDM under defined and controlled conditions, and by means of visual inspection of the state of the steel reinforcement.

(ii) Finding the relation between MER and PDM results and then obtaining a mathematical expression of this relationship.

(iii) Comparison of measured and calculated values.

For the study, mortar test prisms of 40 mm × 40 mm × 160 mm with embedded steel specimens were prepared. The composition of mortar mixtures were as follows: cement : silica sand :: 1 : 3, w/c 0.6 with 1.0, 1.25, 1.5, 2.0, 3.0, and 4.0% doses of calcium chloride added. Percentage values of doses are expressed from the weight of cement portion in the mixes. The prepared mortar test

Table 1. Composition and properties of cement CEM I 42.5.

Chemical composition (%)		Mineralogical composition (according to Bogue) (%)	
Loss ignition	1.77	C ₃ S	48.9
Insoluble residue	1.80	C ₂ S	18.4
SiO ₂	19.28	C ₃ A	12.0
CaO	61.12	C ₄ AF	11.3
Al ₂ O ₃	6.91	CaSO ₄	4.0
Fe ₂ O ₃	3.72	Physical properties	
MgO	1.69		
SO ₃	2.37	Spec. surface area	412 m ² .kg ⁻¹
Na ₂ O	0.31	Spec. weight	3124 kg.m ⁻³
K ₂ O	0.70	Norm. consistency	35%
CaO free	0.40	Initial setting	4 h 30 min
		Setting time	6 h 40 min

Table 2. Results of the study on state of embedded steel specimens in mortars after 100 curing cycles.

Dose of calcium chloride (%)	Visual inspection Extent of corrosion (%)	Electrical resistance method Increase of electrical resistance (μΩ)	Potentiodynamic method Polarization rate (mV.min ⁻¹)		
			30	60	150
			Current density (A.m ⁻²)	Current density (A.m ⁻²)	Current density (A.m ⁻²)
1.00	30	28.9	0.420	0.470	0.280
1.25	45	51.1	1.400	0.880	0.690
1.50	50	62.2	4.050	1.450	0.800
2.00	80	80.0	5.900	2.900	1.400
3.00	90	95.6	7.650	8.200	1.600
4.00	100	111.1	15.000	10.050	3.400

specimens in moulds were cured for 24 h at ca 90% relative humidity and temperature of 20°C. After demoulding, the test specimens were cured under regularly repeated cycles. One cycle represented curing at relative humidity of 75% and 20°C for 16 h, and at 40°C for 8 h. Temperature 40°C was chosen on the basis of our preceding research (Živica *et al* 1997).

Test methods used: For the potentiodynamic measurement standard STN 73 1341 equipment and process were used. MER was used for the measurement of changes of electrical resistance of steel.

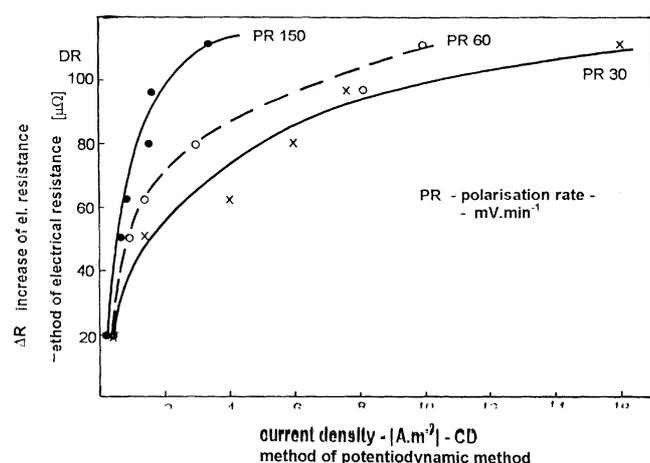


Figure 1. Relationship between values of current density and increase of electrical resistance of steel.

Table 3. Values of experimental coefficient of (2).

Polarization rate (mV·min ⁻¹)	Values of experimental coefficients	
	<i>a</i>	<i>b</i>
30	0.0223	0.0078
60	0.0125	0.0082
150	0.0075	0.0067

Table 4. Measured and calculated values of changes of electrical resistance of embedded steel specimens.

ΔR measured ($\mu\Omega$)	ΔR_c calculated (2)					
	Polarization rate (mV·min ⁻¹)					
	30	Deviation of ΔR_c opposite ΔR (%)	60	Deviation of ΔR_c opposite ΔR (%)	150	Deviation of ΔR_c opposite ΔR (%)
28.9	16.4	-43.3	28.3	-2.1	29.9	+3.5
62.2	74.6	+19.9	58.9	-5.3	62.2	0
80.0	86.4	+8.0	79.5	-0.6	82.9	+3.6
95.6	93.0	+2.7	102.6	+7.3	87.9	-8.1
111.1	107.4	-3.3	108.1	-2.7	103.7	-6.7

After 100 curing cycles, and the destruction of mortar test specimens by visual control, the extent of corrosion of steel test specimens was visually controlled. The extent of corroded area of total steel specimen area was expressed in per cent.

For the preparation of mortars, Portland cement CEM I 42.5 according to standard STN P ENV 197-1 (table 1), and silica sand according to standard STN 72 1208 were used. For the preparation of corrosion sensors construction steel according to standard STN 41 0425 was used. Calcium chloride used was chemically pure.

3. Results and discussion

A summary of the results of visual inspection, as well as MER and PDM results are given in table 2. It is seen that with the increase of calcium chloride doses in the mortar the values of electrical resistance were also increased indicating the increase of corrosion degree of steel. This fact was also visually confirmed. Analysis of the PDM results has shown that of the parameters enabled by this method like stationary potential, passivation potential, break-down potential, and current density, the last parameter showed a good relationship with the mentioned increase of electrical resistance of steel. It is seen that the values of current density increased with the progress of corrosion. Therefore, the dependence of current density on the increase of electrical resistance is shown in figure 1. It is evident that curves differed significantly depending on the polarization rate used at PDM. The increase of polarization rate used caused the decrease of the measured current density. The result was the shortening of the curves and their shift to the higher values of corresponding values of changes of electrical resistance of steel.

For the curves presented in figure 1, following function is relevant

$$\Delta R_c = \frac{CD}{a + b \cdot CD}, \quad (2)$$

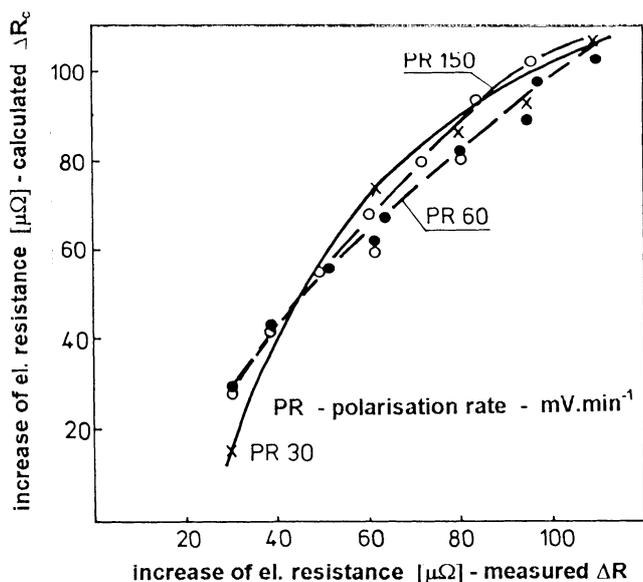


Figure 2. Relationship between values of increase of electrical resistance measured and calculated by means of (2).

where ΔR_c is calculated value of change of electrical resistance of steel in $\mu\Omega$, CD the value of current density in $A \cdot m^{-2}$, a and b are experimental coefficients.

After numbering, the values of the experimental coefficients given in table 3 were found. Then, by means of (2) it is possible to calculate the values of increase of electrical resistance of steel, ΔR_c , using measured current density values for the given polarization rate. The calculated values of ΔR_c are given in table 4. It is seen that when anomaly values of deviation -43.3 and $+19.9\%$ are omitted, the calculated and measured ΔR values show very satisfied correspondence. This is shown in figure 2.

Using the values of ΔR_c it is possible to calculate the reduction of section and material loss of the corroding reinforcement by means of the relationship of MER (Živica 1995, 1997).

4. Conclusions

We show here the possibility to estimate some quantitative data on the corroding steel reinforcement by means of potentiodynamic method and of a calculation approach. This is based on the experimentally found mathematical relationship between potentiodynamically measured current density, and corrosion degree of steel reinforcement measured by means of electrical resistance.

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